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January 3, 2023

Planning Board 900 Main Street Town Offices Millis, MA 02054

Ref: Acorn Place

Stormwater Basin Modifications

Dear Members of the Board:

I am writing on behalf of the applicant to request a modification to the stormwater basin designs at Acorn Place. As has been documented, Basin 1 does not drain down within 72 hours during wet seasons (periods of time with repeated, frequent rainfall events). During these same wet seasons, Basin 2 holds more shallow depths of water for longer than 72 hours. it is well known that the glacial till soils on the site are restrictive¹, which is why there are wetlands on the site in low lying valley areas (areas where stormwater cannot readily runoff). These wetlands exhibit similar patterns to the stormwater basins, holding surface water during wet seasons but being dry during other times of the year.

Due to shallow perched groundwater conditions typically associated with these types of glacial till soils, the design called for unsuitable soils to be removed and the floor of the basin to be filled with clean sand or loamy sand to raise the grade to design elevations. We took samples of the topsoil and underlying fill materials for each basin and had them tested for classification (See Attachment A for results). For Basin 1, a processed gravel material was used as the fill material. Both the loam and processed fill material samples were classified as loamy sand. For Basin 2, Title V septic sand was used as the fill material. Both the loam and processed fill material samples were classified as sand. We therefore conclude that the fill materials imported into the basins are not restricting infiltration.

-

¹ The NRCS soil survey classifies them as Woodbridge, Class C/D (depending on groundwater depth).

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We note the following provisions from the MassDEP Stormwater Handbook relative to stormwater infiltration basins:

- As Volume 1, Chapter 1, page 6-7 notes, "MassDEP recognized that it may be difficult to infiltrate the required recharge volume on certain sites because of soils conditions. For sites comprised solely of C and D soils...proponents are required to infiltrate the required recharge volume only to the maximum extent practicable." It is noted that the site is comprised of Class C and D soils (woodbridge soils per NRCS).
- The minimum permitted design infiltration rate for an infiltration facility is 0.17 inches per hour. Soils with slower permeability rates are typically not deemed to be suitable for infiltration. This is very close to the design infiltration rate for Basin 1 (design rate of 0.4 inches per hour), which supports a conclusion that the natural soils on the site are simply not suited to the infiltration of large volumes of runoff, which is anecdotally supporting by surficial hydrology conditions during certain times of the year.
- ➤ The detail at the top of Volume 2, Chapter 2, page 87 of the Handbook includes the possible provision for an underdrain underneath an infiltration basin "in case of standing water problems."

Given the fact that the site is comprised of C/D soils, during periods of persistent rainfall and perched high groundwater conditions, it is our conclusion that the underlying glacial till is simply unable to accommodate the infiltration of significant volumes of runoff, as the natural till soil matrix is likely already highly saturated by perched groundwater conditions throughout the site during these periods. The performance of Basin 2 supports this conclusion. It is a shallow basin with a holding depth of only 0.3 feet, and it has difficulty draining down within 72 hours during periods of persistent saturation.

While eliminating long periods of standing water with the introduction of a small low-flow outlet at the bottom of each basin is a potential solution, it is our view that the subdrain method noted in the Handbook is likely a better solution. This would allow stormwater residence time in the basin to achieve treatment, will allow some amount of infiltration (as much as the glacial till soils can accommodate, which will vary by season) and will ensure that the basin can drain down within 72 hours. Each of the two basins has a substantial thickness of imported permeable fill soils above the underlying glacial till at the bottom of the basin. It is our recommendation that a subdrain be placed within this permeable fill layer per the enclosed details, which would allow for the slow and gradual release of infiltrated water over a period of days, allowing the basins to draw down in a reasonable time frame.

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As a conservative measure, although water will infiltrate into the underlying permeable fill materials, we have re-run the Hydrocad models with no infiltration accounted for to ensure that there will be no increase in peak rates of runoff. Given the low permeability of the natural glacial till soils, infiltration did not play a significant role in peak rate mitigation. However, due to the specific configuration of the discharge headwalls in the basins, existing outlet configurations in the headwalls would need to be modified to maintain peak rates below pre-development conditions with no infiltration accounted for in the Hydrocad model. See below for further discussion:

DESIGN POINT #1: Flow to Shadowfax Farm

For Basin 1, the elimination of infiltration in the Hydrocad model only resulting in an increased peak rate of runoff in the 100-year storm. Thus, additional low-flow outlets are proposed to change the stage-discharge model to allow all peak rates to be lowered. In order to maintain the Water Quality Volume storage within the basin (4,332 c.f.), three additional 4" diameter cores are proposed to be added to the headwall at elevation 210.80, which results in a raw standing volume of 4,981 c.f. within the basin. These three new outlets will be 0.2 feet lower than the existing low-flow outlet, reducing the depth of standing water in the basin to 0.3 feet. This modification yields the following as indicated in the attached Hydrocad model:

Design	Peak Runoff Rate (cfs)		
Storm (Year)	Existing	Proposed	
1	1.80	1.70	
2	3.78	3.20	
10	9.08	6.83	
50	14.74	10.80	
100	17.30	15.49	

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DESIGN POINT #2: Flow to Southern Abutters

For Basin 2, the elimination of infiltration in the Hydrocad resulting in small peak rate increases in all storm events. Thus, to accommodate the revised stage-storage relationship with no infiltration included, the outlet notch is proposed to be modified per the detail in attachment C. This modification yields the following as indicated in the attached Hydrocad model:

Design	Peak Runoff Rate (cfs)		
Storm (Year)	Existing	Proposed	
1	3.20	3.16	
2	6.64	6.36	
10	15.82	15.32	
50	25.64	25.16	
100	30.09	29.54	

Do not hesitate to contact me should you have any questions or comments.

Yours Truly,

LEGACY ENGINEERING LLC

Daniel J. Merrikin, P.E. President

cc: File

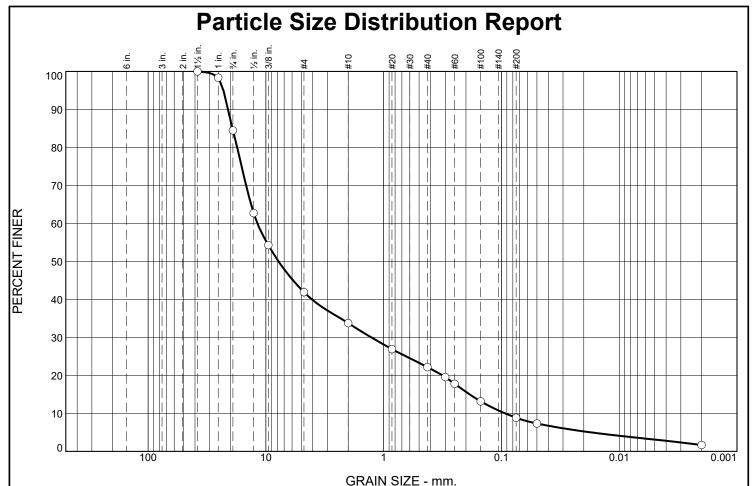
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ATTACHMENT A

STORMWATER BASIN SOILS SAMPLING TEST RESULTS



	OTTAIN OIZE TIIII.								
% Gravel % Sa		% Sand	l	% Fines					
se	Fine	Coarse	Medium	Fine	Silt				
5	42.6	8.1	11.6	13.3	7.2				

SIEVE	PERCENT	SPEC.*	PASS?
SIZE	FINER	PERCENT	(X=NO)
1.5	100.0		
1	98.3		
.75	84.5		
.5	62.7		
.375	54.3		
#4	41.9		
#10	33.8		
#20	26.9		
#40	22.2		
#50	19.5		
#60	17.8		
#100	13.2		
#200	8.9		
0.05mm	7.3		
0.002mm	1.7		

Coarse

15.5

Material Description

Clay

1.7

Brown 1.5" max gravel some f/m sand trace silt USDA Textural Classification = Loamy Sand

Atterberg Limits

PL= NP PI= NP LL= NV

Coefficients

 D_{85} = 19.2385 D_{30} = 1.2719 C_u = 126.95 D₆₀= 11.8080 D₁₅= 0.1863 C_c= 1.47 $D_{50} = 7.8065 \\ D_{10} = 0.0930$

Classification

USCS= GW-GM AASHTO= A-1-a

Remarks

Sample submitted by client on 06/16/22 No soil specs provided

(no specification provided)

% +3"

0.0

Sample No.: L-31524 Source of Sample: Unknown Site **Date:** 6/22/22 Location: Basin Fill #1 Elev./Depth: submitted

YANKEE ENGINEERING & TESTING, INC.

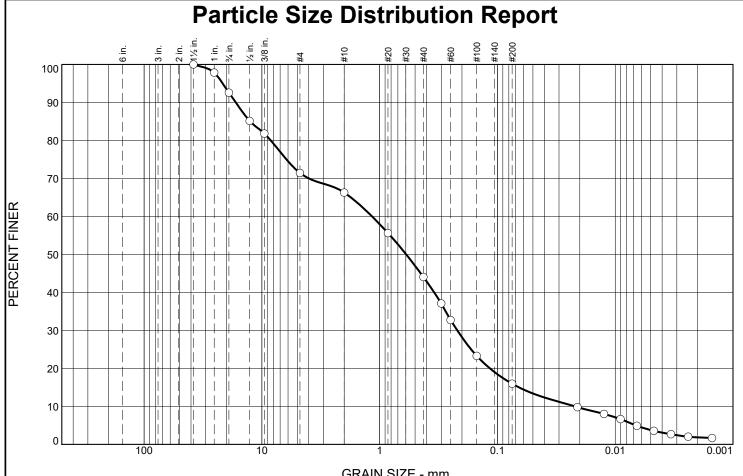
Client: Legacy Engineering

Project: Legacy Engineering BASIN 1 - IMPORTED FILL

Various Projects/Sites

2022.44 Project No:

Tested By: AK Checked By: SMM



GRAIN SIZE - IIIII.							
% +3"	% G	% Gravel % Sand		% Fines			
	Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
0.0	7.4	21.2	5.1	22.3	28.0	14.2	1.8

SIEVE	PERCENT	SPEC.*	PASS?
SIZE	FINER	PERCENT	(X=NO)
1.5	100.0		
1	97.8		
.75	92.6		
.5	85.1		
.375	81.8		
#4	71.4		
#10	66.3		
#20	55.6		
#40	44.0		
#50	37.1		
#60	32.7		
#100	23.3		
#200	16.0		

Material Description

Brown 1.5" max silty sand some gravel USDA Textural Classification = Loamy Sand

 $\begin{array}{ccc} & & \underline{\text{Atterberg Limits}} \\ \text{PL= NP} & & \underline{\text{LL= NV}} & & \text{Pl= NP} \end{array}$

 $\begin{array}{c|cccc} & & & & & & & \\ D_{85} = & 12.5906 & & D_{60} = & 1.1609 & & D_{50} = & 0.5955 \\ D_{30} = & 0.2193 & & D_{15} = & 0.0656 & & D_{10} = & 0.0220 \\ C_u = & 52.78 & & C_c = & 1.88 & & & & & \\ \end{array}$

USCS= SM Classification AASHTO= A-1-b

Remarks

Sample submitted by client on 06/16/22 No soil specs provided

Silty soils = moisture susceptible

(no specification provided)

Sample No.: L-31525 Source of Sample: Unknown Site Date: 6/22/22 Location: Topsoil #1 Elev./Depth: submitted

YANKEE ENGINEERING & TESTING, INC.

Client: Legacy Engineering

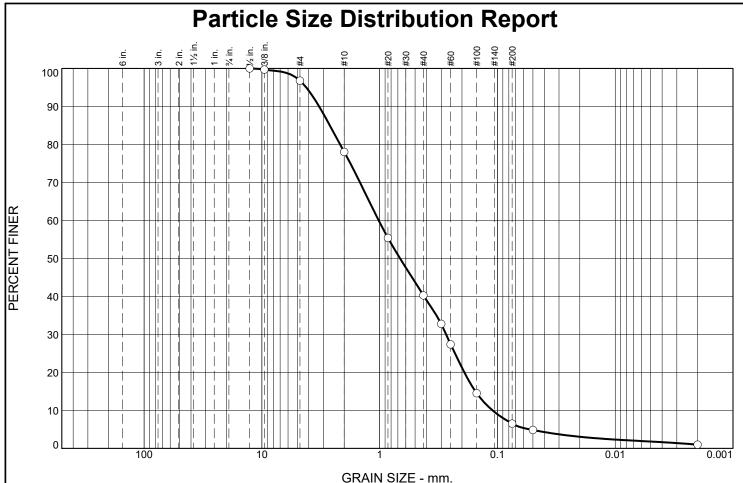
Project: Legacy Engineering

BASIN 1 - TOPSOIL

Various Projects/Sites

Project No: 2022.44

Tested By: AK / AH Checked By: SMM



GRAIN SIZE - IIIIII.							
% +3"	% G	ravel	avel % Sand		% Fines		
	Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
0.0	0.0	3.2	18.8	37.7	33.8	5.5	1.0

SIEVE	PERCENT	SPEC.*	PASS?
SIZE	FINER	PERCENT	(X=NO)
.5	100.0		
.375	99.7		
#4	96.8		
#10	78.0		
#20	55.4		
#40	40.3		
#50	32.8		
#60	27.4		
#100	14.5		
#200	6.5		
0.05mm	4.8		
0.002mm	1.0		

Material Description

Brown m/f sand trace silt trace gravel USDA Textural Classification = Sand

Atterberg Limits

USCS= SP-SM Classification
AASHTO= A-1-b

Remarks
Sample submitted by client on 06/16/22
No soil specs provided

(no specification provided)

Sample No.: L-31526 Source of Sample: Unknown Site Date: 6/22/22 Location: Basin Fill #2 Elev./Depth: submitted

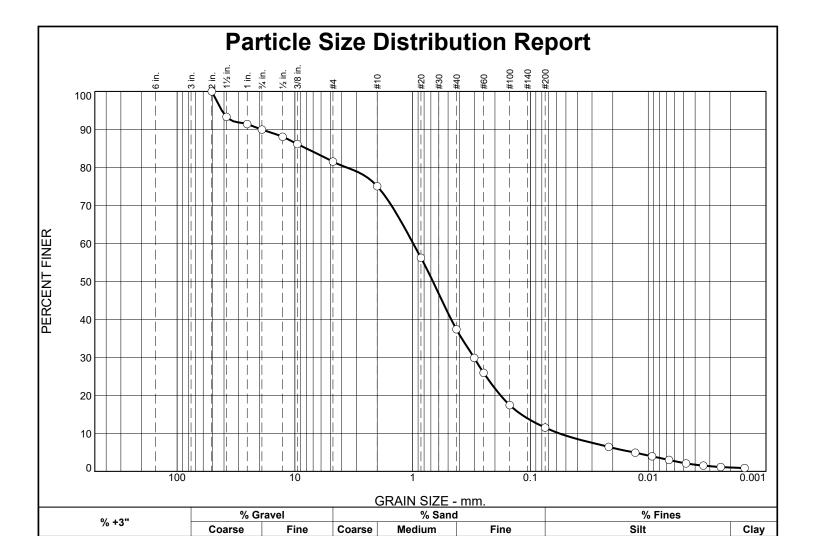
YANKEE ENGINEERING & TESTING, INC.

Client: Legacy Engineering

Project: Legacy Engineering BASIN 2 - IMPORTED FILL Various Projects/Sites

Project No: 2022.44

Tested By: AK / AH Checked By: SMM



37.7

SIEVE	PERCENT	SPEC.*	PASS?
SIZE	FINER	PERCENT	(X=NO)
2	100.0		
1.5	93.3		
1	91.4		
.75	90.0		
.5	88.1		
.375	86.1		
#4	81.5		
#10	75.1		
#20	56.2		
#40	37.4		
#50	29.9		
#60	25.9		
#100	17.4		
#200	11.5		

10.0

8.5

6.4

Material Description

Brown 1.5" max silty m/f sand little gravel USDA Textural Classification = Sand

25.9

PL= NP	Atterberg Limits LL= NV	PI= NP
D ₈₅ = 7.9974 D ₃₀ = 0.3020 C _u = 17.41	$\begin{array}{c} {\hbox{\tt Coefficients}} \\ {\hbox{\tt D}_{60}} = \ 0.9909 \\ {\hbox{\tt D}_{15}} = \ 0.1198 \\ {\hbox{\tt C}_{c}} = \ 1.62 \\ \\ {\hbox{\tt Classification}} \end{array}$	D ₅₀ = 0.6734 D ₁₀ = 0.0569

10.5

1.0

USCS= SW-SM AASHTO= A-1-b

Remarks client on 06/16/22

Sample submitted by client on 06/16/22 No soil specs provided

(no specification provided)

0.0

Sample No.: L-31527 Source of Sample: Unknown Site Date: 6/22/22 Location: Topsoil #2 Elev./Depth: submitted

YANKEE ENGINEERING & TESTING, INC.

Client: Legacy Engineering

Project: Legacy Engineering Various Projects/Sites BASIN 2 - TOPSOIL

Project No: 2022.44

Tested By: AK / AH Checked By: SMM

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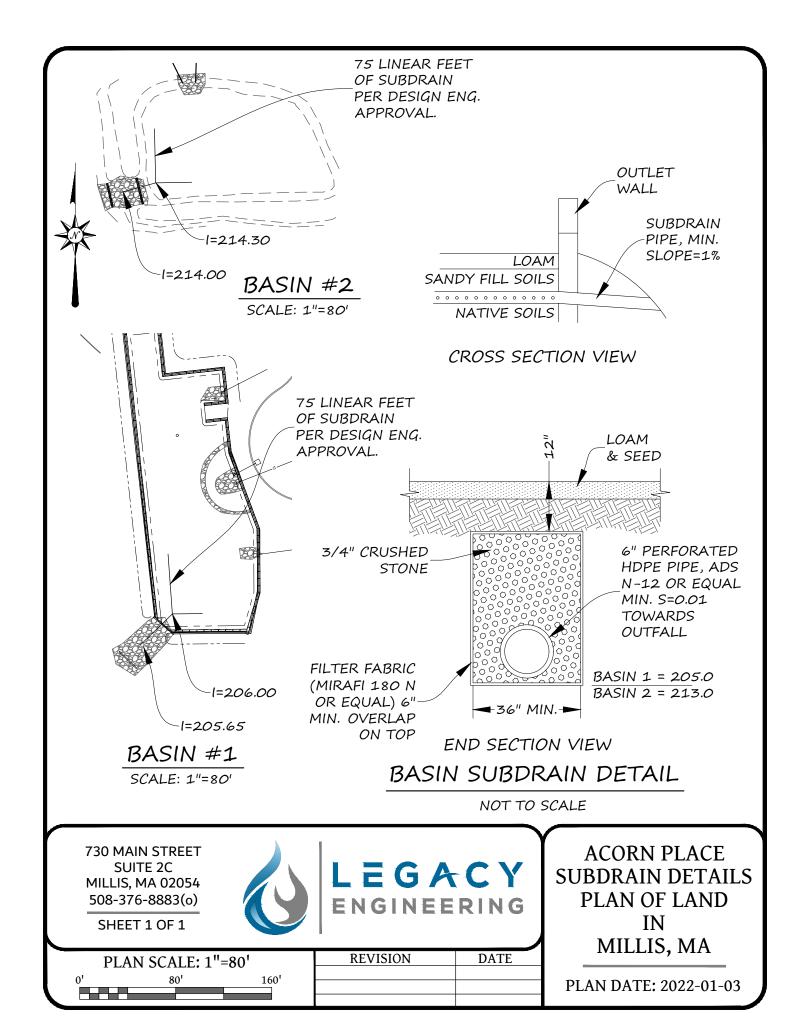


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Millis, MA 02054

ATTACHMENT B

BASIN SUBDRAIN DETAILS



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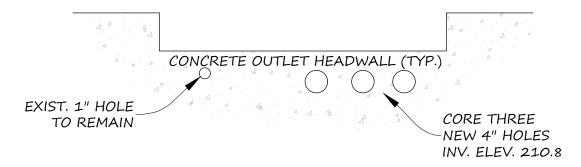


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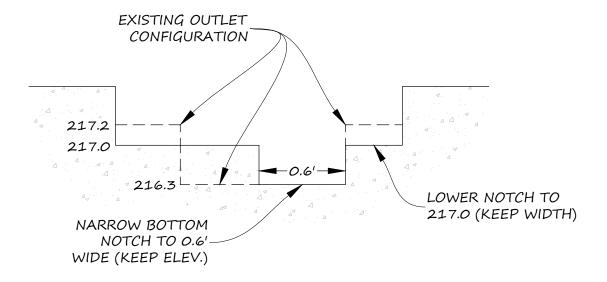
ATTACHMENT C

BASIN HEADWALL MODIFICATION DETAILS



BASIN #1 OUTLET DETAIL

NOT TO SCALE



BASIN #2 OUTLET DETAIL

NOT TO SCALE

730 MAIN STREET SUITE 2C MILLIS, MA 02054 508-376-8883(o)

08-376-8883(o) SHEET 1 OF 1



PLAN SCALE: 1"=80'			REVISION	DATE
0'	80'	160'		
	80	100		

ACORN PLACE HEADWALL DETAIL PLAN OF LAND IN MILLIS, MA

PLAN DATE: 2022-01-03

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ATTACHMENT D

HYDROCAD RESULTS – NO BASIN INFILTRATION

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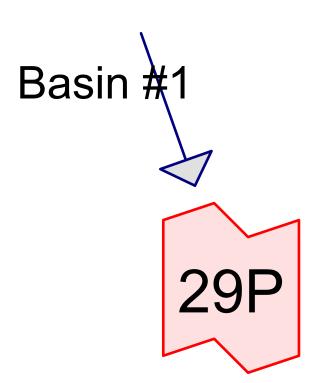


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Millis, MA 02054

DESIGN POINT #1 - BASIN 1





Design Point #1: Flow to Shadowfax Farm









2023-01-03 Basin Reconfiguration

Type III 24-hr 1-YR Rainfall=2.50" Printed 1/3/2023

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Page 2

Time span=0.00-30.00 hrs, dt=0.01 hrs, 3001 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Stor-Ind method - Pond routing by Stor-Ind method

Peak Elev=211.27' Storage=13,436 cf Inflow=6.17 cfs 0.536 af Pond 22P: Basin #1

Outflow=0.71 cfs 0.398 af

Inflow=1.70 cfs 0.541 af Link 29P: Design Point #1: Flow to Shadowfax Farm

Primary=1.70 cfs 0.541 af

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Printed 1/3/2023 Page 3

Summary for Pond 22P: Basin #1

Inflow Area = 3.658 ac, 77.31% Impervious, Inflow Depth = 1.76" for 1-YR event

Inflow = 6.17 cfs @ 12.12 hrs, Volume= 0.536 af

Outflow = 0.71 cfs @ 12.99 hrs, Volume= 0.398 af, Atten= 88%, Lag= 52.3 min

Primary = 0.71 cfs @ 12.99 hrs, Volume= 0.398 af Routed to Link 29P : Design Point #1: Flow to Shadowfax Farm

Routing by Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Peak Elev= 211.27' @ 12.99 hrs Surf.Area= 18,041 sf Storage= 13,436 cf

Plug-Flow detention time= 307.0 min calculated for 0.398 af (74% of inflow)

Center-of-Mass det. time= 218.9 min (1,017.8 - 799.0)

<u>Volume</u>	Inv	<u>ert Avai</u>	l.Storage	Storage Descripti	on		
#1	210.	50' 5	53,991 cf	Custom Stage D	ata (Irregular) List	ed below (Recalc)	
Elevation		Surf.Area	Perim.	Inc.Store	Cum.Store	Wet.Area	
(fee	et)	(sq-ft)	(feet)	(cubic-feet)	(cubic-feet)	<u>(sq-ft)</u>	
210.5	50	16,003	665.0	0	0	16,003	
211.0	00	18,041	690.0	8,506	8,506	18,720	
213.0	00	18,041	690.0	36,082	44,588	20,100	
213.5	50	19,580	704.0	9,403	53,991	21,692	
Device	Routing	lnv	vert Outle	et Devices			
#1	Primary	212.	.32' Cus t	tom Weir/Orifice,	Cv= 2.62 (C= 3.28	3)	
	•		Elev	. (feet) 212.32 2	13.00	•	
			Widt	h (feet) 15.00 15.	00		
#2	Primary	211.	.00' 1.0"	Vert. Orifice/Grat	e C= 0.600 Lim	ited to weir flow at low hea	ds
#3	Primary	210.		Vert. Orifice/Grat ted to weir flow at I		0	

Primary OutFlow Max=0.71 cfs @ 12.99 hrs HW=211.27' (Free Discharge)

—1=Custom Weir/Orifice (Controls 0.00 cfs)

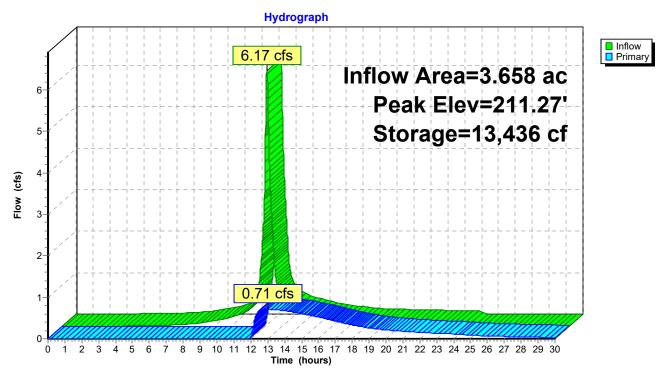
—2=Orifice/Grate (Orifice Controls 0.01 cfs @ 2.32 fps)

-3=Orifice/Grate (Orifice Controls 0.70 cfs @ 2.67 fps)

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Pond 22P: Basin #1



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Summary for Link 29P: Design Point #1: Flow to Shadowfax Farm

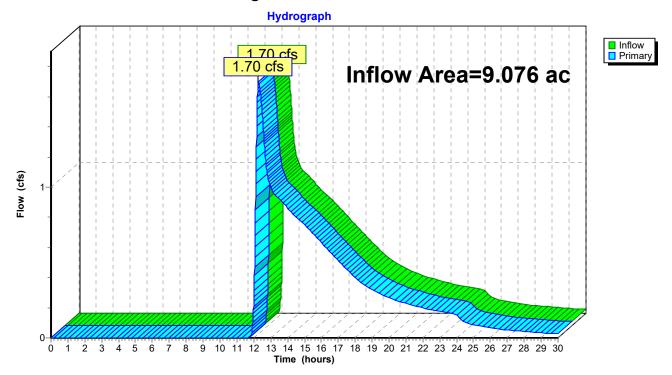
Inflow Area = 9.076 ac, 41.50% Impervious, Inflow Depth > 0.71" for 1-YR event

Inflow = 1.70 cfs @ 12.27 hrs, Volume= 0.541 af

Primary = 1.70 cfs @ 12.27 hrs, Volume= 0.541 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs

Link 29P: Design Point #1: Flow to Shadowfax Farm



2023-01-03 Basin ReconfigurationPrepared by Legacy Engineering LLC

Type III 24-hr 2-YR Rainfall=3.20" Printed 1/3/2023

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Page 6

Time span=0.00-30.00 hrs, dt=0.01 hrs, 3001 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Stor-Ind method - Pond routing by Stor-Ind method

Pond 22P: Basin #1 Peak Elev=211.55' Storage=18,448 cf Inflow=8.40 cfs 0.736 af

Outflow=0.98 cfs 0.596 af

Link 29P: Design Point #1: Flow to Shadowfax Farm Inflow=3.20 cfs 0.851 af

Primary=3.20 cfs 0.851 af

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Summary for Pond 22P: Basin #1

Inflow Area = 3.658 ac, 77.31% Impervious, Inflow Depth = 2.42" for 2-YR event

Inflow = 8.40 cfs @ 12.12 hrs, Volume= 0.736 af

Outflow = 0.98 cfs @ 12.96 hrs, Volume= 0.596 af, Atten= 88%, Lag= 50.1 min

Primary = 0.98 cfs @ 12.96 hrs, Volume= 0.596 af Routed to Link 29P : Design Point #1: Flow to Shadowfax Farm

Routing by Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Peak Elev= 211.55' @ 12.96 hrs Surf.Area= 18,041 sf Storage= 18,448 cf

Plug-Flow detention time= 297.4 min calculated for 0.596 af (81% of inflow)

Center-of-Mass det. time= 223.2 min (1,014.8 - 791.6)

<u>Volume</u>	Inv	<u>ert Avai</u>	l.Storage	Storage Descripti	on				
#1	210.	50' 5	53,991 cf	Custom Stage D	ata (Irregular) List	ed below (Recalc)			
Elevation		Surf.Area	Perim.	Inc.Store	Cum.Store	Wet.Area			
(fee	et)	(sq-ft)	(feet)	(cubic-feet)	(cubic-feet)	<u>(sq-ft)</u>			
210.5	50	16,003	665.0	0	0	16,003			
211.0	00	18,041	690.0	8,506	8,506	18,720			
213.00		18,041	690.0	36,082	44,588	20,100			
213.5	50	19,580	704.0	9,403	53,991	21,692			
Device	Routing	lnv	vert Outle	et Devices					
#1	Primary	212.	.32' Cus t	tom Weir/Orifice,	Cv= 2.62 (C= 3.28	3)			
	•		Elev	. (feet) 212.32 2	13.00	•			
			Widt	h (feet) 15.00 15.	00				
#2	Primary	211.	.00' 1.0"	1.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low head					
#3 Primary 210.80' 4.0" Vert. Orifice/Grate X 3.00 C= 0.600 Limited to weir flow at low heads									

Primary OutFlow Max=0.98 cfs @ 12.96 hrs HW=211.55' (Free Discharge)

—1=Custom Weir/Orifice (Controls 0.00 cfs)

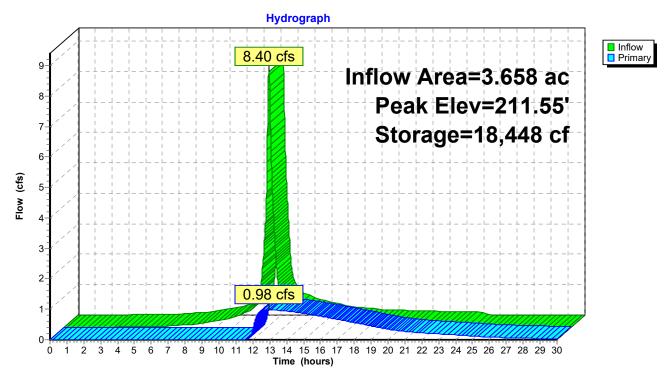
—2=Orifice/Grate (Orifice Controls 0.02 cfs @ 3.44 fps)

-3=Orifice/Grate (Orifice Controls 0.96 cfs @ 3.68 fps)

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Pond 22P: Basin #1



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Summary for Link 29P: Design Point #1: Flow to Shadowfax Farm

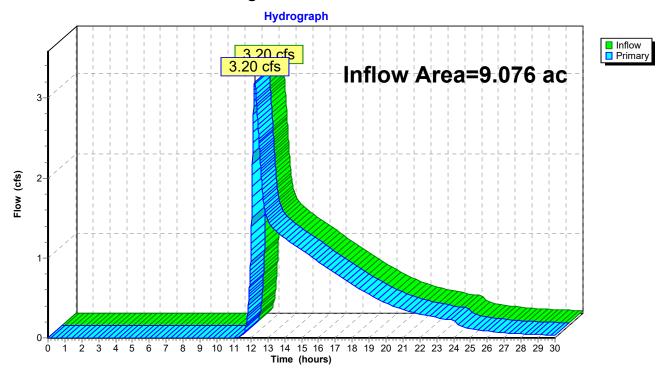
Inflow Area = 9.076 ac, 41.50% Impervious, Inflow Depth > 1.13" for 2-YR event

Inflow = 3.20 cfs @ 12.23 hrs, Volume= 0.851 af

Primary = 3.20 cfs @ 12.23 hrs, Volume= 0.851 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs

Link 29P: Design Point #1: Flow to Shadowfax Farm



2023-01-03 Basin Reconfiguration

Type III 24-hr 10-YR Rainfall=4.70"

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Time span=0.00-30.00 hrs, dt=0.01 hrs, 3001 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Stor-Ind method - Pond routing by Stor-Ind method

Pond 22P: Basin #1 Peak Elev=212.16' Storage=29,472 cf Inflow=13.20 cfs 1.176 af

Outflow=1.41 cfs 1.030 af

Link 29P: Design Point #1: Flow to Shadowfax Farm Inflow=6.83 cfs 1.582 af

Primary=6.83 cfs 1.582 af

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Summary for Pond 22P: Basin #1

Inflow Area = 3.658 ac, 77.31% Impervious, Inflow Depth = 3.86" for 10-YR event

Inflow = 13.20 cfs @ 12.12 hrs, Volume= 1.176 af

Outflow = 1.41 cfs @ 13.02 hrs, Volume= 1.030 af, Atten= 89%, Lag= 53.8 min

Primary = 1.41 cfs @ 13.02 hrs, Volume= 1.030 af Routed to Link 29P : Design Point #1: Flow to Shadowfax Farm

Routing by Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Peak Elev= 212.16' @ 13.02 hrs Surf.Area= 18,041 sf Storage= 29,472 cf

Plug-Flow detention time= 307.7 min calculated for 1.030 af (88% of inflow)

Center-of-Mass det. time= 250.9 min (1,031.8 - 780.9)

Volume	Inv	ert Avai	I.Storage	Storage Descripti	on		
#1	210.	50'	53,991 cf	Custom Stage D	ata (Irregular)List	ed below (Recalc)	
Elevation	on	Surf.Area	Perim.	Inc.Store	Cum.Store	Wet.Area	
(fee	et)	(sq-ft)	(feet)	(cubic-feet)	(cubic-feet)	(sq-ft)	
210.5	50	16,003	665.0	0	0	16,003	
211.0	00	18,041	690.0	8,506	8,506	18,720	
213.0	00	18,041	690.0	36,082	44,588	20,100	
213.5	50	19,580	704.0	9,403	53,991	21,692	
Device Routing Invert Outlet Devices							
#1	Primary	212	.32' Cus t	tom Weir/Orifice,	Cv= 2.62 (C= 3.28	3)	
Elev. (feet) 212.32 213.00							
			Widt	h (feet) 15.00 15.	.00		
#2	Primary	211.	.00' 1.0"	Vert. Orifice/Grat	e C= 0.600 Limi	ited to weir flow at low head	st
#3	#3 Primary 210.80' 4.0" Vert. Orifice/Grate X 3.00 C= 0.600						
			Limit	ted to weir flow at I	ow heads		

Primary OutFlow Max=1.41 cfs @ 13.02 hrs HW=212.16' (Free Discharge)

—1=Custom Weir/Orifice (Controls 0.00 cfs)

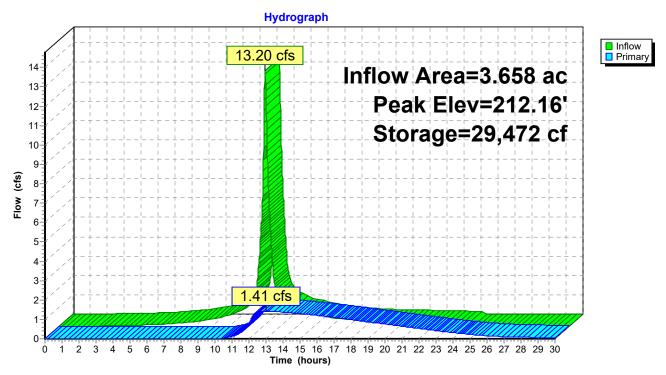
—2=Orifice/Grate (Orifice Controls 0.03 cfs @ 5.10 fps)

-3=Orifice/Grate (Orifice Controls 1.38 cfs @ 5.26 fps)

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Pond 22P: Basin #1



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Summary for Link 29P: Design Point #1: Flow to Shadowfax Farm

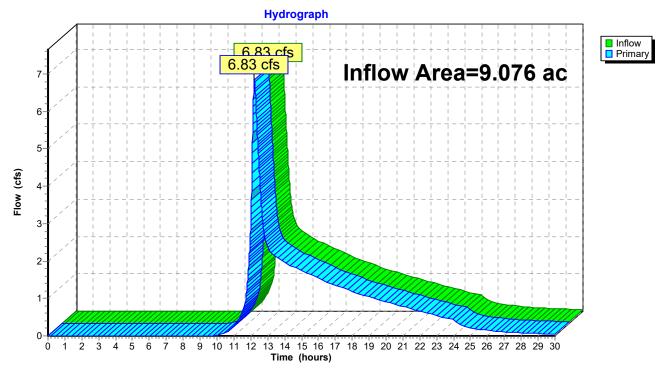
Inflow Area = 9.076 ac, 41.50% Impervious, Inflow Depth > 2.09" for 10-YR event

Inflow = 6.83 cfs @ 12.21 hrs, Volume= 1.582 af

Primary = 6.83 cfs @ 12.21 hrs, Volume= 1.582 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs

Link 29P: Design Point #1: Flow to Shadowfax Farm



2023-01-03 Basin Reconfiguration

Type III 24-hr 50-YR Rainfall=6.10" Printed 1/3/2023

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Time span=0.00-30.00 hrs, dt=0.01 hrs, 3001 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Stor-Ind method - Pond routing by Stor-Ind method

Pond 22P: Basin #1 Peak Elev=212.50' Storage=35,609 cf Inflow=17.65 cfs 1.594 af

Outflow=5.42 cfs 1.443 af

Link 29P: Design Point #1: Flow to Shadowfax Farm Inflow=10.80 cfs 2.345 af

Primary=10.80 cfs 2.345 af

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Summary for Pond 22P: Basin #1

Inflow Area = 3.658 ac, 77.31% Impervious, Inflow Depth = 5.23" for 50-YR event

Inflow = 17.65 cfs @ 12.12 hrs, Volume= 1.594 af

Outflow = 5.42 cfs @ 12.50 hrs, Volume= 1.443 af, Atten= 69%, Lag= 22.9 min

Primary = 5.42 cfs @ 12.50 hrs, Volume= 1.443 af
Routed to Link 29P: Design Point #1: Flow to Shadowfax Farm

Routing by Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Peak Elev= 212.50' @ 12.50 hrs Surf.Area= 18,041 sf Storage= 35,609 cf

Plug-Flow detention time= 279.1 min calculated for 1.443 af (91% of inflow)

Center-of-Mass det. time= 232.0 min (1,006.2 - 774.2)

<u>Volume</u>	Inv	<u>ert Avai</u>	l.Storage	Storage Descripti	on				
#1	210.	50' 5	53,991 cf	Custom Stage D	ata (Irregular) List	ed below (Recalc)			
Elevation		Surf.Area	Perim.	Inc.Store	Cum.Store	Wet.Area			
(fee	et)	(sq-ft)	(feet)	(cubic-feet)	(cubic-feet)	<u>(sq-ft)</u>			
210.5	50	16,003	665.0	0	0	16,003			
211.0	00	18,041	690.0	8,506	8,506	18,720			
213.00		18,041	690.0	36,082	44,588	20,100			
213.5	50	19,580	704.0	9,403	53,991	21,692			
Device	Routing	lnv	vert Outle	et Devices					
#1	Primary	212.	.32' Cus t	tom Weir/Orifice,	Cv= 2.62 (C= 3.28	3)			
	•		Elev	. (feet) 212.32 2	13.00	•			
			Widt	h (feet) 15.00 15.	00				
#2	Primary	211.	.00' 1.0"	1.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low head					
#3 Primary 210.80' 4.0" Vert. Orifice/Grate X 3.00 C= 0.600 Limited to weir flow at low heads									

Primary OutFlow Max=5.42 cfs @ 12.50 hrs HW=212.50' (Free Discharge)

1=Custom Weir/Orifice (Weir Controls 3.82 cfs @ 1.40 fps)

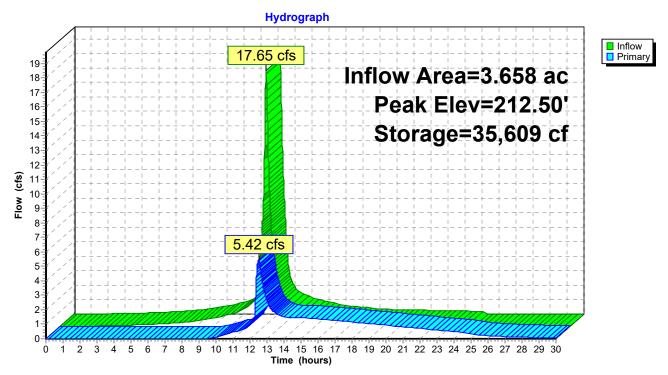
—2=Orifice/Grate (Orifice Controls 0.03 cfs @ 5.82 fps)

-3=Orifice/Grate (Orifice Controls 1.56 cfs @ 5.97 fps)

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Pond 22P: Basin #1



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Summary for Link 29P: Design Point #1: Flow to Shadowfax Farm

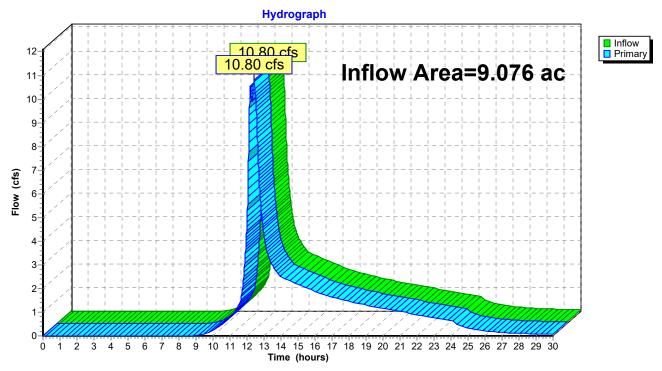
Inflow Area = 9.076 ac, 41.50% Impervious, Inflow Depth > 3.10" for 50-YR event

Inflow = 10.80 cfs @ 12.42 hrs, Volume= 2.345 af

Primary = 10.80 cfs @ 12.42 hrs, Volume= 2.345 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs

Link 29P: Design Point #1: Flow to Shadowfax Farm



2023-01-03 Basin Reconfiguration

Type III 24-hr 100-YR Rainfall=6.70"

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Time span=0.00-30.00 hrs, dt=0.01 hrs, 3001 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Stor-Ind method - Pond routing by Stor-Ind method

Pond 22P: Basin #1 Peak Elev=212.57' Storage=36,885 cf Inflow=19.55 cfs 1.774 af

Outflow=7.89 cfs 1.622 af

Link 29P: Design Point #1: Flow to Shadowfax Farm Inflow=15.49 cfs 2.700 af

Primary=15.49 cfs 2.700 af

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Summary for Pond 22P: Basin #1

Inflow Area = 3.658 ac, 77.31% Impervious, Inflow Depth = 5.82" for 100-YR event

Inflow = 19.55 cfs @ 12.12 hrs, Volume= 1.774 af

Outflow = 7.89 cfs @ 12.42 hrs, Volume= 1.622 af, Atten= 60%, Lag= 17.9 min

Primary = 7.89 cfs @ 12.42 hrs, Volume= 1.622 af Routed to Link 29P : Design Point #1: Flow to Shadowfax Farm

Routing by Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Peak Elev= 212.57' @ 12.42 hrs Surf.Area= 18,041 sf Storage= 36,885 cf

Plug-Flow detention time= 262.1 min calculated for 1.621 af (91% of inflow)

Center-of-Mass det. time= 218.3 min (990.1 - 771.9)

<u>Volume</u>	Inv	<u>ert Avai</u>	l.Storage	Storage Descripti	on				
#1	210.	50' 5	53,991 cf	Custom Stage D	ata (Irregular) List	ed below (Recalc)			
Elevation		Surf.Area	Perim.	Inc.Store	Cum.Store	Wet.Area			
(fee	et)	(sq-ft)	(feet)	(cubic-feet)	(cubic-feet)	<u>(sq-ft)</u>			
210.5	50	16,003	665.0	0	0	16,003			
211.0	00	18,041	690.0	8,506	8,506	18,720			
213.00		18,041	690.0	36,082	44,588	20,100			
213.5	50	19,580	704.0	9,403	53,991	21,692			
Device	Routing	lnv	vert Outle	et Devices					
#1	Primary	212.	.32' Cus t	tom Weir/Orifice,	Cv= 2.62 (C= 3.28	3)			
	•		Elev	. (feet) 212.32 2	13.00	•			
			Widt	h (feet) 15.00 15.	00				
#2	Primary	211.	.00' 1.0"	1.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low head					
#3 Primary 210.80' 4.0" Vert. Orifice/Grate X 3.00 C= 0.600 Limited to weir flow at low heads									

Primary OutFlow Max=7.88 cfs @ 12.42 hrs HW=212.57' (Free Discharge)

1=Custom Weir/Orifice (Weir Controls 6.25 cfs @ 1.65 fps)

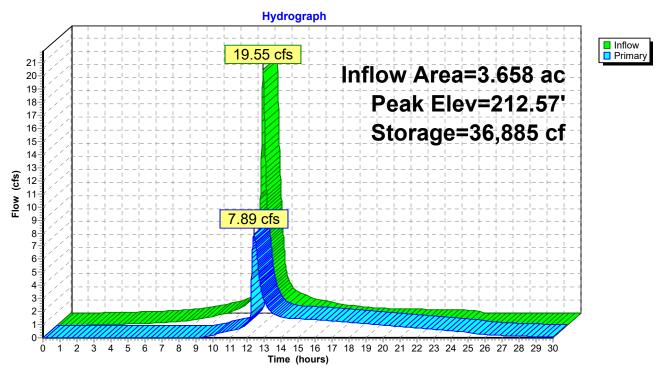
—2=Orifice/Grate (Orifice Controls 0.03 cfs @ 5.96 fps)

-3=Orifice/Grate (Orifice Controls 1.60 cfs @ 6.10 fps)

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Pond 22P: Basin #1



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Summary for Link 29P: Design Point #1: Flow to Shadowfax Farm

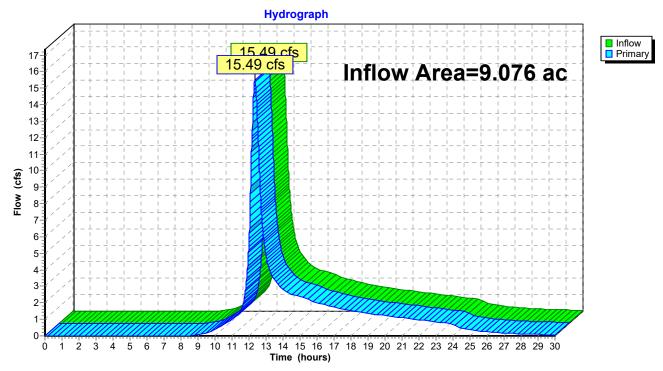
Inflow Area = 9.076 ac, 41.50% Impervious, Inflow Depth > 3.57" for 100-YR event

Inflow = 15.49 cfs @ 12.34 hrs, Volume= 2.700 af

Primary = 15.49 cfs @ 12.34 hrs, Volume= 2.700 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs

Link 29P: Design Point #1: Flow to Shadowfax Farm



Planning Board January 3, 2023 Page 10 of 10



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Millis, MA 02054

DESIGN POINT #2 - BASIN 2







Design Point #2: Flow to Southern Abutters









Type III 24-hr 1-YR Rainfall=2.50" Printed 1/3/2023

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Time span=0.00-30.00 hrs, dt=0.01 hrs, 3001 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Stor-Ind method - Pond routing by Stor-Ind method

Pond 43P: Basin #2 Peak Elev=216.69' Storage=8,859 cf Inflow=3.40 cfs 0.356 af

Outflow=0.47 cfs 0.255 af

Link 44P: Design Point #2: Flow to Southern Abutters Inflow=3.16 cfs 0.683 af

Primary=3.16 cfs 0.683 af

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Summary for Pond 43P: Basin #2

Inflow Area = 2.747 ac, 65.33% Impervious, Inflow Depth = 1.56" for 1-YR event

Inflow = 3.40 cfs @ 12.16 hrs, Volume= 0.356 af

Outflow = 0.47 cfs @ 13.06 hrs, Volume= 0.255 af, Atten= 86%, Lag= 54.3 min

Primary = 0.47 cfs @ 13.06 hrs, Volume= 0.255 af Routed to Link 44P : Design Point #2: Flow to Southern Abutters

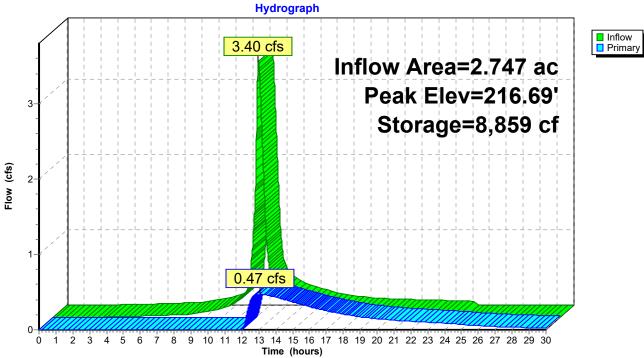
Routing by Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Peak Elev= 216.69' @ 13.06 hrs Surf.Area= 13,557 sf Storage= 8,859 cf

Plug-Flow detention time= 326.1 min calculated for 0.255 af (72% of inflow) Center-of-Mass det. time= 231.6 min (1,038.3 - 806.7)

Volume	Inve	rt Avai	il.Storage	Storage Descripti	on		
#1	216.0	0'	36,749 cf	Custom Stage D	ata (Irregular)List	ted below (Recalc)	1
Elevatio		Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
216.00 218.50		12,268 17,274	457.0 522.0	0 36,749	0 36,749	12,268 17,478	
Device	Routing	ng Invert Outlet Devices					
#1	#1 Primary 216.30' Custom Weir/Orifice, Cv= 2.62 (C= 3.28) Elev. (feet) 216.30 217.00 217.00 218.00 Width (feet) 0.60 0.60 2.00 2.00						

Primary OutFlow Max=0.47 cfs @ 13.06 hrs HW=216.69' (Free Discharge) 1=Custom Weir/Orifice (Weir Controls 0.47 cfs @ 2.04 fps)

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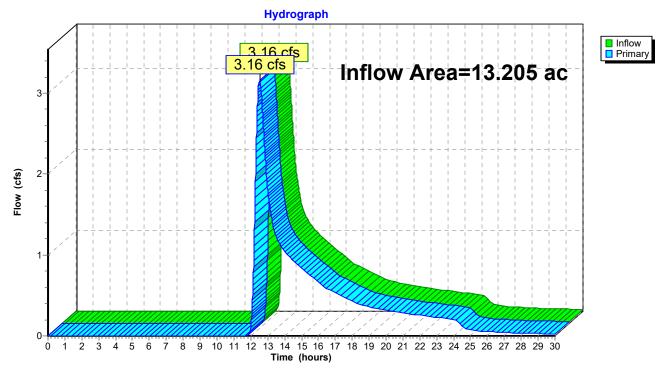
Summary for Link 44P: Design Point #2: Flow to Southern Abutters

Inflow Area = 13.205 ac, 13.59% Impervious, Inflow Depth > 0.62" for 1-YR event

Inflow = 3.16 cfs @ 12.53 hrs, Volume= 0.683 af

Primary = 3.16 cfs @ 12.53 hrs, Volume= 0.683 af, Atten= 0%, Lag= 0.0 min

Link 44P: Design Point #2: Flow to Southern Abutters



Type III 24-hr 2-YR Rainfall=3.20" Printed 1/3/2023

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Time span=0.00-30.00 hrs, dt=0.01 hrs, 3001 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Stor-Ind method - Pond routing by Stor-Ind method

Pond 43P: Basin #2 Peak Elev=216.90' Storage=11,859 cf Inflow=4.74 cfs 0.499 af

Outflow=0.92 cfs 0.395 af

Link 44P: Design Point #2: Flow to Southern Abutters Inflow=6.36 cfs 1.161 af

Primary=6.36 cfs 1.161 af

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Summary for Pond 43P: Basin #2

Inflow Area = 2.747 ac, 65.33% Impervious, Inflow Depth = 2.18" for 2-YR event

Inflow = 4.74 cfs @ 12.16 hrs, Volume= 0.499 af

Outflow = 0.92 cfs @ 12.80 hrs, Volume= 0.395 af, Atten= 81%, Lag= 38.4 min

Primary = 0.92 cfs @ 12.80 hrs, Volume= 0.395 af Routed to Link 44P : Design Point #2: Flow to Southern Abutters

Routing by Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Peak Elev= 216.90' @ 12.80 hrs Surf.Area= 13,980 sf Storage= 11,859 cf

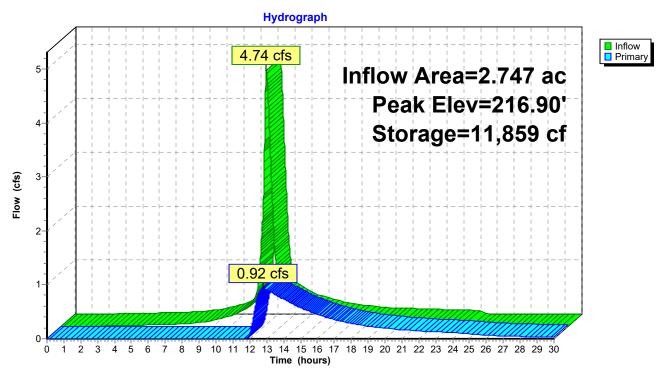
Plug-Flow detention time= 280.3 min calculated for 0.395 af (79% of inflow)

Center-of-Mass det. time= 201.5 min (1,001.8 - 800.3)

Volume	Inve	rt Avai	I.Storage	Storage Description	on		
#1	216.0	0'	36,749 cf	Custom Stage Da	ata (Irregular)List	ed below (Recalc)	
Elevatio (feet		Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
216.0 218.5		12,268 17,274	457.0 522.0	0 36,749	0 36,749	12,268 17,478	
Device	Routing	Routing Invert Outlet Devices					
#1 Primary 216.30' Custom Weir/Orifice, Cv= 2.62 (C= 3.28) Elev. (feet) 216.30 217.00 217.00 218.00 Width (feet) 0.60 0.60 2.00 2.00							

Primary OutFlow Max=0.92 cfs @ 12.80 hrs HW=216.90' (Free Discharge) 1=Custom Weir/Orifice (Weir Controls 0.92 cfs @ 2.55 fps)

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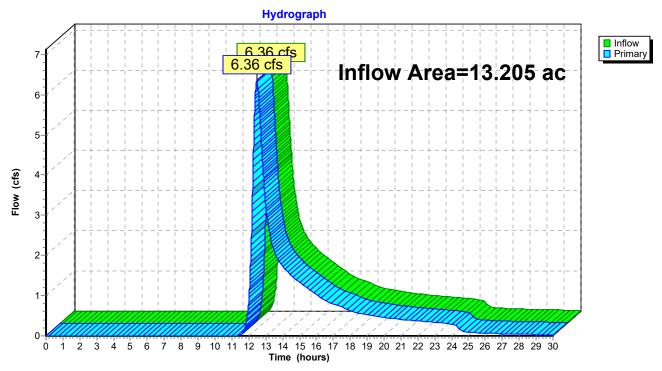
Summary for Link 44P: Design Point #2: Flow to Southern Abutters

Inflow Area = 13.205 ac, 13.59% Impervious, Inflow Depth > 1.05" for 2-YR event

Inflow = 6.36 cfs @ 12.47 hrs, Volume= 1.161 af

Primary = 6.36 cfs @ 12.47 hrs, Volume= 1.161 af, Atten= 0%, Lag= 0.0 min

Link 44P: Design Point #2: Flow to Southern Abutters



2023-01-03 Basin ReconfigurationPrepared by Legacy Engineering LLC

Type III 24-hr 10-YR Rainfall=4.70" Printed 1/3/2023

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Time span=0.00-30.00 hrs, dt=0.01 hrs, 3001 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Stor-Ind method - Pond routing by Stor-Ind method

Pond 43P: Basin #2 Peak Elev=217.29' Storage=17,430 cf Inflow=7.63 cfs 0.818 af

Outflow=2.67 cfs 0.712 af

Link 44P: Design Point #2: Flow to Southern Abutters Inflow=15.32 cfs 2.361 af

Primary=15.32 cfs 2.361 af

Type III 24-hr 10-YR Rainfall=4.70"

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Summary for Pond 43P: Basin #2

Inflow Area = 2.747 ac, 65.33% Impervious, Inflow Depth = 3.57" for 10-YR event

7.63 cfs @ 12.16 hrs, Volume= Inflow = 0.818 af

2.67 cfs @ 12.61 hrs, Volume= Outflow = 0.712 af, Atten= 65%, Lag= 27.1 min

Primary = 2.67 cfs @ 12.61 hrs, Volume= 0.712 af, 2.67 cfs @ 12.61 hrs, Volume= 0.712 af Routed to Link 44P: Design Point #2: Flow to Southern Abutters

Routing by Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Peak Elev= 217.29' @ 12.61 hrs Surf.Area= 14,749 sf Storage= 17,430 cf

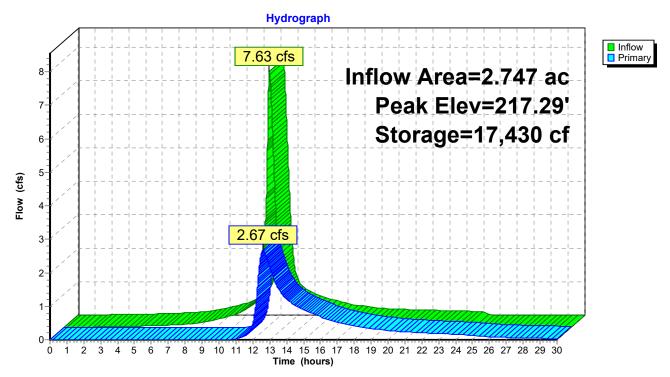
Plug-Flow detention time= 221.6 min calculated for 0.712 af (87% of inflow)

Center-of-Mass det. time= 163.1 min (953.6 - 790.4)

Volume	Inve	rt Ava	il.Storage	Storage Description	on		
#1	216.0	0'	36,749 cf	Custom Stage D	ata (Irregular) List	ed below (Recalc)	
Elevatio (fee		Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
216.0 218.5	-	12,268 17,274	457.0 522.0	0 36,749	0 36,749	12,268 17,478	
Device	Routing	ting Invert Outlet Devices					
#1 Primary 216.30' Custom Weir/Orifice, Cv= 2.62 (C= 3.28) Elev. (feet) 216.30 217.00 218.00 Width (feet) 0.60 0.60 2.00 2.00							

Primary OutFlow Max=2.67 cfs @ 12.61 hrs HW=217.29' (Free Discharge) 1=Custom Weir/Orifice (Weir Controls 2.67 cfs @ 2.65 fps)

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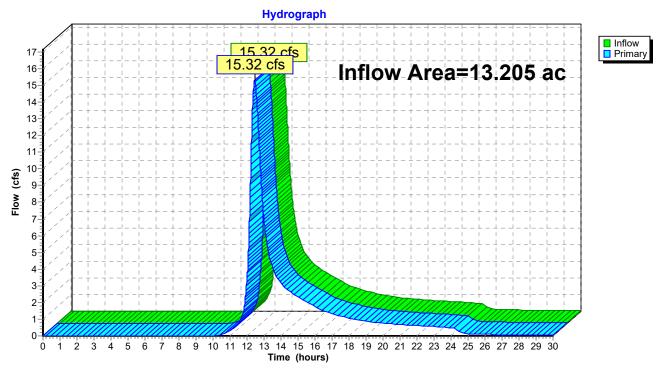
Summary for Link 44P: Design Point #2: Flow to Southern Abutters

Inflow Area = 13.205 ac, 13.59% Impervious, Inflow Depth > 2.15" for 10-YR event

Inflow = 15.32 cfs @ 12.46 hrs, Volume= 2.361 af

Primary = 15.32 cfs @ 12.46 hrs, Volume= 2.361 af, Atten= 0%, Lag= 0.0 min

Link 44P: Design Point #2: Flow to Southern Abutters



Type III 24-hr 50-YR Rainfall=6.10"

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Time span=0.00-30.00 hrs, dt=0.01 hrs, 3001 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Stor-Ind method - Pond routing by Stor-Ind method

Pond 43P: Basin #2 Peak Elev=217.56' Storage=21,440 cf Inflow=10.27 cfs 1.124 af

Outflow=4.69 cfs 1.017 af

Link 44P: Design Point #2: Flow to Southern Abutters Inflow=25.16 cfs 3.613 af

Primary=25.16 cfs 3.613 af

Type III 24-hr 50-YR Rainfall=6.10"

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Summary for Pond 43P: Basin #2

Inflow Area = 2.747 ac, 65.33% Impervious, Inflow Depth = 4.91" for 50-YR event

Inflow = 10.27 cfs @ 12.16 hrs, Volume= 1.124 af

Outflow = 4.69 cfs @ 12.54 hrs, Volume= 1.017 af, Atten= 54%, Lag= 22.9 min

Primary = 4.69 cfs @ 12.54 hrs, Volume= 1.017 af Routed to Link 44P : Design Point #2: Flow to Southern Abutters

Routing by Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Peak Elev= 217.56' @ 12.54 hrs Surf.Area= 15,290 sf Storage= 21,440 cf

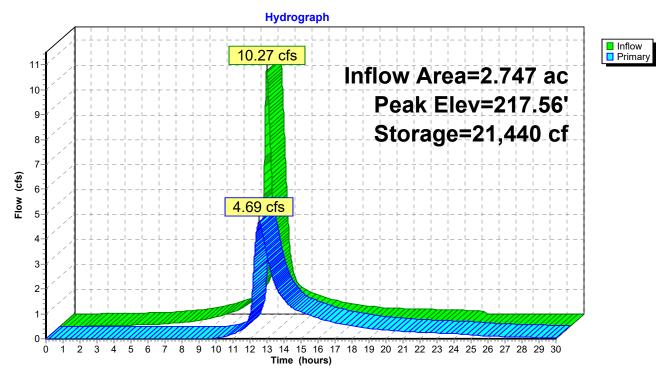
Plug-Flow detention time= 188.2 min calculated for 1.017 af (90% of inflow)

Center-of-Mass det. time= 140.6 min (924.6 - 784.0)

Volume	Inve	<u>rt Avai</u>	I.Storage	Storage Descripti	on		
#1	216.0	0'	36,749 cf	Custom Stage D	ata (Irregular) List	ed below (Recalc)	
Elevatio (feet		Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
216.0 218.5		12,268 17,274	457.0 522.0	0 36,749	0 36,749	12,268 17,478	
Device	Routing	Routing Invert Outlet Devices					
#1 Primary 216.30' Custom Weir/Orifice, Cv= 2.62 (C= 3.28) Elev. (feet) 216.30 217.00 218.00 Width (feet) 0.60 0.60 2.00 2.00							

Primary OutFlow Max=4.69 cfs @ 12.54 hrs HW=217.56' (Free Discharge) 1=Custom Weir/Orifice (Weir Controls 4.69 cfs @ 3.05 fps)

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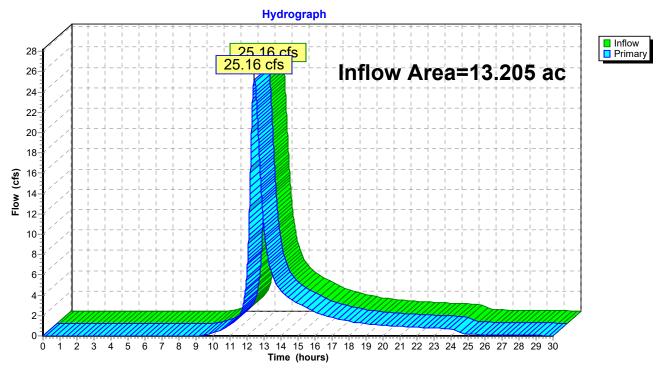
Summary for Link 44P: Design Point #2: Flow to Southern Abutters

Inflow Area = 13.205 ac, 13.59% Impervious, Inflow Depth > 3.28" for 50-YR event

Inflow = 25.16 cfs @ 12.43 hrs, Volume= 3.613 af

Primary = 25.16 cfs @ 12.43 hrs, Volume= 3.613 af, Atten= 0%, Lag= 0.0 min

Link 44P: Design Point #2: Flow to Southern Abutters



Type III 24-hr 100-YR Rainfall=6.70"

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Time span=0.00-30.00 hrs, dt=0.01 hrs, 3001 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Stor-Ind method - Pond routing by Stor-Ind method

Pond 43P: Basin #2 Peak Elev=217.66' Storage=22,977 cf Inflow=11.36 cfs 1.257 af

Outflow=5.57 cfs 1.149 af

Link 44P: Design Point #2: Flow to Southern Abutters Inflow=29.54 cfs 4.175 af

Primary=29.54 cfs 4.175 af

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Summary for Pond 43P: Basin #2

Inflow Area = 2.747 ac, 65.33% Impervious, Inflow Depth = 5.49" for 100-YR event

Inflow = 11.36 cfs @ 12.16 hrs, Volume= 1.257 af

Outflow = 5.57 cfs @ 12.52 hrs, Volume= 1.149 af, Atten= 51%, Lag= 21.9 min

Primary = 5.57 cfs @ 12.52 hrs, Volume= 1.149 af Routed to Link 44P : Design Point #2: Flow to Southern Abutters

Routing by Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Peak Elev= 217.66' @ 12.52 hrs Surf.Area= 15,495 sf Storage= 22,977 cf

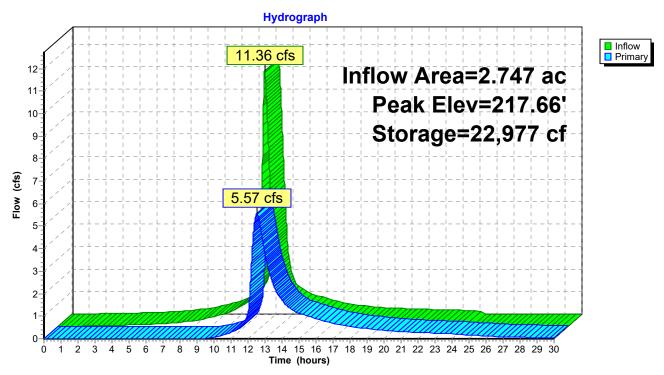
Plug-Flow detention time= 177.8 min calculated for 1.149 af (91% of inflow)

Center-of-Mass det. time= 133.7 min (915.5 - 781.8)

Volume	Inve	<u>rt Avai</u>	l.Storage	Storage Description	on		
#1	216.0	0' 3	36,749 cf	Custom Stage Da	ata (Irregular)Liste	ed below (Recalc)	
Elevatio	n	Surf.Area	Perim.	Inc.Store	Cum.Store	Wet.Area	
(feet	t)	(sq-ft)	(feet)	(cubic-feet)	(cubic-feet)	(sq-ft)	
216.0	0	12,268	457.0	0	0	12,268	
218.5	0	17,274	522.0	36,749	36,749	17,478	
Device	Routing	uting Invert Outlet Devices					
#1	Primary	216.30' Custom Weir/Orifice, Cv= 2.62 (C= 3.28)					
Elev. (feet) 216.30 217.00 218.00							
			Widtl	h (feet) 0.60 0.60	2.00 2.00		

Primary OutFlow Max=5.57 cfs @ 12.52 hrs HW=217.66' (Free Discharge) 1=Custom Weir/Orifice (Weir Controls 5.57 cfs @ 3.20 fps)

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Summary for Link 44P: Design Point #2: Flow to Southern Abutters

Inflow Area = 13.205 ac, 13.59% Impervious, Inflow Depth > 3.79" for 100-YR event

Inflow = 29.54 cfs @ 12.43 hrs, Volume= 4.175 af

Primary = 29.54 cfs @ 12.43 hrs, Volume= 4.175 af, Atten= 0%, Lag= 0.0 min

Link 44P: Design Point #2: Flow to Southern Abutters

